#### 1. GENERAL

- PROVIDE CONSTRUCTION CONFORMING TO THE 2018 INTERNATIONAL BUILDING CODE WITH THE 2020 GEORGIA AMENDMENTS. REFERENCE TO OTHER STANDARDS, SPECIFICATIONS, OR CODES MEANS THE LATEST STANDARD OR CODE PUBLISHED AND ADOPTED
- THE STRUCTURAL GENERAL NOTES APPLY EXCEPT WHERE INDICATED OTHERWISE ON THE DRAWINGS OR IN THE SPECIFICATIONS. A DETAIL SHOWN FOR ONE CONDITION APPLIES FOR ALL LIKE OR SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY INDICATED ON THE DRAWINGS.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, AND ALL OTHER MEANS, METHODS, TECHNIQUES, SEQUENCES, AND
- PROCEDURES OF CONSTRUCTION. COORDINATE THE STRUCTURAL CONTRACT DOCUMENTS WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CIVIL, AND ALL OTHER CONSULTANTS. NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD IN WRITING OF ANY CONFLICT AND/OR OMISSION.
- CONTRACTOR SHALL LOCATE ALL BURIED UTILITIES PRIOR TO EXCAVATION FOR BUILDING FOUNDATIONS. THE STRUCTURAL ENGINEER SHALL BE NOTIFIED OF POTENTIAL CONFLICTS BETWEEN FOUNDATIONS AND BURIED
- COORDINATE AND VERIFY FLOOR AND ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. FOR ADDITIONAL OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS REFER TO THE ARCHITECTURAL AND MECHANICAL DRAWINGS.

- SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT & ENGINEER OF RECORD PRIOR TO FABRICATION AND CONSTRUCTION REGARDING ALL STRUCTURAL ITEMS INCLUDING THE FOLLOWING:
- CONCRETE MIX DESIGNS
- CONCRETE REINFORCING
- STRUCTURAL STEEL FRAMING STEEL DECK
- IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF GEORGIA. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT AND ARE SUBJECT TO THE REVIEW AND ACCEPTANCE BY THE ENGINEER OF RECORD
- REVIEW OF THE SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER OF RECORD IS ONLY FOR GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS AND DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD. THE CONTRACTOR MUST REVIEW AND STAMP ALL SUBMITTALS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD.
- DO NOT MAKE SHOP DRAWINGS USING REPRODUCTIONS OF THE CONTRACT DOCUMENTS OR REFERENCING THE CONTRACT DOCUMENTS.
- DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS, SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF GEORGIA, AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION. SHOP DRAWINGS MUST INDICATE THE REQUIRED MATERIALS, SIZES, AND LOCATIONS OF ALL FRAMING MEMBERS AND CONNECTIONS. CALCULATIONS SHALL BE INCLUDED FOR ALL CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON THE STRUCTURAL ELEMENTS INDUCED BY THE CONNECTION LOADS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE EDITION OUTLINED IN STRUCTURAL GENERAL NOTE SECTION 1. THE ITEMS REQUIRED TO BE DESIGNED BY OTHERS INCLUDES:
- COLD FORMED LIGHT GAGE METAL FRAMING
- CURTAIN WALLS AND ALL OTHER GLAZING SYSTEMS PRE-ENGINEERED METAL BUILDING SYSTEM
- PROVIDE CURTAIN WALL GLAZING SYSTEM SHOP DRAWINGS THAT CLEARLY INDICATE THE ATTACHMENT TO THE STRUCTURE ON ALL SIDES OF THE EXTERIOR GLAZING SYSTEM REQUIRED TO ADEQUATELY RESIST THE APPLICABLE WIND DESIGN PRESSURES.
- THE GLAZING CONTRACTOR MUST PROVIDE ENGINEERING CALCULATIONS TO DOCUMENT COMPLIANCE WITH THE LATEST EDITION OF THE INTERNATIONAL BUILDING CODE, SECTIONS 2403.2 THROUGH 2303.4 FOR BUTT JOINED
- THE DESIGN OF SPECIAL CONNECTIONS BETWEEN STEEL FRAMING COMPONENTS (INCLUDING BUT NOT LIMITED TO BRACED END CONNECTIONS, MOMENT-RESISTING CONNECTIONS, MODIFIED BEAM SEAT CONNECTIONS, AND MEMBER SPLICE CONNECTIONS) NOT DESIGNED BY THE STRUCTURAL ENGINEER OF RECORD MUST BE PERFORMED. BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA.

- PROVIDE REINFORCED CONCRETE CONFORMING TO THE FOLLOWING STANDARDS:
- ACI 301-11, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS ACI 318-11, BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
- ACI 302.1R-15, GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION ACI 360R-10, DESIGN OF SLABS-ON-GROUND
- UNLESS NOTED OTHERWISE, PROVIDE NORMAL WEIGHT CONCRETE WITH 3,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.
- PROVIDE 4% TO 6% ENTRAINED AIR BY VOLUME IN CONCRETE PERMANENTLY EXPOSED TO WEATHER.
- PROVIDE CONCRETE WITH A MAXIMUM WATER-TO-CEMENTITIOUS MATERIALS RATIO OF 0.50. FULLY DOCUMENT AND SUBMIT FOR REVIEW THE PROPOSED MATERIALS AND MIX DESIGN FOR ALL CONCRETE.
- THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING THE REQUIRED DESIGN STRENGTH. ALL CONCRETE TEST
- DATA MUST BE AVAILABLE AT THE JOB SITE. THE USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IS NOT PERMITTED.
- PLACE CONCRETE AT A SLUMP OF  $5'' \pm 1''$ .
- THE FINAL LOCATION OF CONSTRUCTION JOINTS REQUIRES THE APPROVAL OF THE STRUCTURAL ENGINEER OF RECORD. UNLESS NOTED OTHERWISE, THOROUGHLY ROUGHEN (BY MECHANICAL MEANS) AND CLEAN CONSTRUCTION JOINTS AND PLACE @ 15'-0" ON CENTER. CHAMFER OR ROUND ALL EXPOSED CORNERS A MINIMUM OF 3/4".
- DETAIL CONCRETE REINFORCEMENT ACCORDING TO ACI SP-66 DETAILING MANUAL. SUBMIT SHOP DRAWINGS FOR APPROVAL SHOWING ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING CONCRETE REINFORCING AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD.
- UNLESS NOTED OTHERWISE, PROVIDE REINFORCING STEEL CONFORMING TO ASTM A615, GRADE 60. PROVIDE WELDED WIRE FABRIC (MESH) IN FLAT SHEETS (ROLLS NOT PERMITTED) CONFORMING TO ASTM A185 AND ASTM A 82. LAP WELDED WIRE FABRIC A MINIMUM OF 6" AT EACH SPLICE. PLACE WELDED WIRE FABRIC 1"
- BELOW THE TOP OF SLABS-ON-GRADE. PROTECTIVE MOISTURE BARRIER IS TO BE INSTALLED BELOW ALL SLABS ON GRADE, SEE PLANS FOR MINIMUM
- REQUIRED THICKNESS OF VAPOR BARRIER AND PROVIDE MIN. 6" LAP. TIE ALL REINFORCING STEEL AND EMBEDDED ITEMS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF THE REINFORCEMENT WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS, ANCHOR RODS, OR OTHER EMBEDDED ITEMS INTO
- WET CONCRETE IS NOT PERMITTED PROVIDE CORNER BARS AT ALL CORNERS AND INTERSECTIONS OF FOOTINGS. LAP CORNER BARS WITH MAIN
- REINFORCING AS OUTLINED IN REINFORCING LAP SLICE SCHEDULE THIS SHEET. PROVIDE BASIC CLASS "B" TENSION LAPS IN ALL REINFORCING BARS INDICATED AS CONTINUOUS.
- THE PLACEMENT OF ALL REINFORCING STEEL MUST BE REVIEWED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA OR BY A REPRESENTATIVE RESPONSIBLE TO HIM (REF: ACI 318, 1.3.1).
- UNLESS NOTED OTHERWISE, PROVIDE THE FOLLOWING CONCRETE COVER ON ALL REINFORCING STEEL
- FOUNDATIONS (NOT FORMED): 3" BOTTOM AND SIDES, 2" TOP
- 1" TOP DO NOT PLACE PIPES OR DUCTS WITH A MAXIMUM DIMENSION EXCEEDING ONE-THIRD THE SLAB THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON THE STRUCTURAL DRAWINGS.
- DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE STRUCTURAL ENGINEER OF RECORD. PROVIDE REINFORCING STEEL CONFORMING TO ASTM A706, GRADE 60 WHERE WELDING
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE AN ALLOWANCE OF REINFORCING BARS TO BE PLACED DURING PROGRESS OF WORK AS MAY BE DIRECTED BY THE STRUCTURAL ENGINEER OF RECORD DURING CONSTRUCTION IN ADDITION TO ALL REINFORCING STEEL INDICATED ON THE CONTRACT DOCUMENTS.

### 4. COLD FORMED METAL FRAMING

- PROVIDE COLD-FORMED METAL FRAMING CONFORMING WITH THE FOLLOWING STANDARDS: NAS-01, AISI NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL
- PROVIDE STUDS, RUNNER TRACK, AND ASSOCIATED ACCESSORIES OF THE TYPE AND THICKNESS INDICATED ON THE DRAWINGS OR AS RECOMMENDED BY THE MANUFACTURER FOR THE INDICATED APPLICATION.
- PROVIDE STUDS, JOISTS, RUNNER TRACK, AND ACCESSORIES MANUFACTURED OF HOT DIP GALVANIZED ASTM A 1003 STEEL WITH THE FOLLOWING YIELD STRENGTH:
- 33,000 PSI 33-MIL (20 GAGE) AND 43-MIL (18 GAGE) MEMBERS 50,000 PSI 54-MIL (16 GAGE), 68-MIL (14 GAGE), AND 97-MIL (12 GAGE) MEMBERS
- PROVIDE MINIMUM S-12 SCREWS (CADMIUM OR ZINC COATED).
- INSTALL ALL COLD-FORMED METAL FRAMING IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS PROVIDE REINFORCEMENT FOR ANY MEMBER CUT FOR THE INSTALLATION OF PLUMBING OR WIRING SUCH THAT
- THE MEMBER IS OF EOUAL STRENGTH TO THE MEMBER PRIOR TO CUTTING. PROVIDE LATERAL BRIDGING IN LOAD BEARING WALLS CONSISTING OF 1 1/2" COLD-ROLLED CHANNELS AT 4'-0" ON CENTER VERTICALLY. INSERT CHANNELS THROUGH STUD WEB HOLES AND SCREW OR WELD TO EACH STUD USING 1 1/2" X 1" X 54-MIL (16 GAGE) CLIP ANGLES. PROVIDE CLIP ANGLES 1/4" LESS THAN THE STUD WIDTH.
- PROVIDE JOIST BRIDGING AT 8'-0" ON CENTER MAXIMUM. INSTALL JOIST BRIDGING IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- PROVIDE UNPUNCHED MATERIAL FOR ALL JOISTS AND HEADERS.
- PROVIDE SOLID BLOCKING BEHIND ALL HORIZONTAL PANEL JOINTS OF EXTERIOR WALL SHEATHING.
- DESIGN OF EXTERIOR WALLS SUBJECT TO WIND PRESSURES IS BASED ON LATERAL BRACING OF STUD FLANGES PROVIDED BY SHEATHING. IF SHEATHING IS NOT APPLIED TO BOTH FACES OF THE WALL, PROVIDE BRACING IN ACCORDANCE WITH SSMA TECHNICAL NOTE NO. 2, PUBLISHED MARCH 2005.

#### **5. FOUNDATIONS**

- THE OWNER MUST COMMISSION A GEOTECHNICAL EXPLORATION OF THE SITE BY A PROPERLY INSURED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA AND FORWARD THE GEOTECHNICAL ENGINEER'S REPORT TO THE STRUCTURAL ENGINEER OF RECORD. THE GEOTECHNICAL ENGINEER'S REPORT MUST CONFORM TO SECTION 1802.6 OF THE 2012 INTERNATIONAL BUILDING CODE. THE DESIGN OF FOUNDATIONS IS BASED ON THE FOLLOWING ASSUMED SOIL CRITERIA: ALLOWABLE SOIL BEARING PRESSURE: 2,000 PSF
- SOIL DENSITY: 110 PCF REDESIGN OF FOUNDATIONS MY BE REQUIRED IF THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT ARE DIFFERENT THAN THE VALUES LISTED ABOVE. THE FOLLOWING CONDITIONS COULD ALSO RESULT IN REDESIGN OF FOUNDATIONS: PRESENCE OF EXPANSIVE SOILS, HIGH WATER TABLE, POTENTIAL FOR LARGE SETTLEMENTS, OR ANY OTHER RECOMMENDATIONS STATED IN THE GEOTECHNICAL ENGINEER'S REPORT.
- IF ANY INTERFERENCE APPEARS BETWEEN EXISTING FOUNDATIONS AND THE SPECIFIED DESIGN, NOTIFY THE ARCHITECT SO THAT THE FOUNDATIONS MAY BE REDESIGNED AS REQUIRED.
- CENTER ALL FOOTINGS AND PIERS UNDER COLUMNS ABOVE UNLESS SPECIFICALLY DIMENSIONED OTHERWISE. PLACE ALL COLUMN FOOTINGS AND WALL FOOTINGS MONOLITHICALLY WITH ADJACENT FOOTINGS AT THE SAME
- THE GEOTECHNICAL ENGINEER MUST VERIFY THE CONDITION AND/OR ADEQUACY OF ALL SUBGRADES, FILLS, AND BACKFILLS PRIOR TO THE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, ETC.
- COORDINATE TOP OF FOOTING ELEVATIONS WITH THE REQUIREMENTS OF OTHER TRADES (PLUMBING,
- ALL FOOTINGS MUST BEAR ON ORIGINAL UNDISTURBED SOIL WHERE POSSIBLE. REMOVE ALL ORGANIC SOILS AND REPLACE WITH CLEAN STRUCTURAL FILL AT THE DIRECTION OF THE GEOTECHNICAL ENGINEER. PLACE FILL SOILS IN 6" MAXIMUM (LOOSE) LIFTS AT MOISTURE CONTENTS AS DESCRIBED IN THE GEOTECHNICAL REPORT. COMPACT ALL FILL WITHIN 10'-0" OF THE BUILDING LIMIT TO 95% STANDARD PROCTOR. FIELD DENSITY TESTS MUST BE MADE AS DESCRIBED IN THE GEOTECHNICAL REPORT TO
- VERIFY ADEQUATE COMPACTION AND DESIGN BEARING PRESSURE. SIDES OF FOUNDATIONS MUST BE FORMED UNLESS CONDITIONS PERMIT EARTH FORMING. FOUNDATIONS PLACED AGAINST THE EARTH REQUIRE THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS AS APPROVED BY THE GEOTECHNICAL ENGINEER AND CLEAN UP SLOUGHING BEFORE AND DURING CONCRETE
- WHERE FOOTING STEPS ARE NECESSARY, SLOPE NO STEEPER THAN ONE VERTICAL TO TWO HORIZONTAL.

#### 6. STRUCTURAL STEEL

ELECTRICAL, ETC.).

- PROVIDE STRUCTURAL STEEL CONFORMING TO THE FOLLOWING STANDARDS: AISC MANUAL OF STEEL CONSTRUCTION, 14TH EDITION
- AISC 360-10, SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AISC 303-10, CODE OF STANDARD PRACTICE FOR STRUCTURAL STEEL BUILDINGS AND BRIDGES
- AISC 326-09, DETAILING FOR STEEL CONSTRUCTION, 2ND EDITION UNLESS NOTED OTHERWISE, PROVIDE STEEL SHAPES MADE OF MATERIAL CONFORMING TO THE FOLLOWING
  - STANDARDS WIDE FLANGE: ASTM A992, GRADE 50 •
  - ANGLES, PLATES, AND CHANNELS: ASTM A36 HOLLOW STRUCTURAL SECTIONS (HSS): ASTM A500, GRADE B (FY=46 KSI)
- **ASTM F1554, GRADE 36** UNLESS NOTED OTHERWISE, MAKE ALL BOLTED SHEAR CONNECTIONS WITH 3/4" DIAMETER ASTM A325 BOLTS. ASSEMBLE AND INSPECT BOLTED CONNECTIONS IN ACCORDANCE WITH AISC "SPECIFICATION FOR JOINTS USING
- ASTM A 325 OR ASTM A 490 BOLTS". MAKE ALL WELDED CONNECTIONS IN ACCORDANCE WITH AWS D1.1-11 "STRUCTURAL WELDING CODE", USING TYPE E70XX ELECTRODES AND SHALL BE 3/16" MINIMUM UNLESS OTHERWISE NOTED OR DETAILED. USE ONLY CERTIFIED WELDERS. PROOF OF CERTIFICATION MUST BE MAINTAINED AT THE JOB SITE.
- SUBMIT SHOP DRAWINGS PREPARED IN ACCORDANCE WITH AISC 326-09. PROVIDE COMPLETE WELDING INFORMATION USING AWS SYMBOLS. USE PREQUALIFIED WELDED JOINTS PER AISC AND AWS D1.1 "STRUCTURAL WELDING CODE." DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED BY THE
- STRUCTURAL ENGINEER OF RECORD DO NOT USE GAS CUTTING TORCHES TO CORRECT FABRICATION ERRORS IN STRUCTURAL STEEL FRAMING. PROVIDE TEMPORARY BRACING FOR STRUCTURAL STEEL FRAMING UNTIL ALL PERMANENT BRACING, MOMENT
- CONNECTIONS, AND FLOOR/ROOF DECKS (DIAPHRAGMS) ARE COMPLETELY INSTALLED. PAINT STRUCTURAL STEEL IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS. DO NOT PAINT STEEL SURFACES TO BE ENCASED IN CONCRETE, SURFACES TO RECEIVE FIREPROOFING, CONNECTIONS DESIGNATED AS FRICTION TYPE, SURFACES TO BE WELDED, OR SURFACES RECEIVING WELDED STUDS OR DBA'S IN THE FIELD.
- IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE AN ALLOWANCE OF STEEL FRAMING TO BE PLACED DURING PROGRESS OF WORK AS MAY BE DIRECTED BY THE STRUCTURAL ENGINEER OF RECORD DURING CONSTRUCTION IN ADDITION TO ALL STEEL FRAMING INDICATED ON THE CONTRACT DOCUMENTS.

#### 7. STEEL DECKING

- PROVIDE FABRICATION AND ERECTION OF STEEL DECKING CONFORMING TO THE FOLLOWING STANDARDS: SDI DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS
- PROVIDE STEEL DECK OF THE TYPE AND GAGE INDICATED ON THE DESIGN DOCUMENTS STEEL ROOF DECK SHALL BE GALVANIZED AND CONFORM TO ASTM A653, STRUCTURAL QUALITY. THE GALVANIZED COATING SHALL CONFORM TO ASTM A653 G60; WHERE LEFT PERMANENTLY EXPOSED TO WEATHER THE COATING SHALL BE G90. ATTACHMENTS, CLOSURES ETC. SHALL BE IN ACCORDANCE WITH MANUFACTURERS
- RECOMMENDATIONS. INSTALL STEEL DECKING IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATION. DO NOT INSTALL STEEL DECKING UNTIL SUPPORTING JOISTS ARE BRACED, BRIDGED, AND PERMANENTLY FASTENED.
- INSTALL DECKING SUCH THAT IT IS CONTINUOUS OVER A MINIMUM OF THREE SPANS. MINIMUM DECK GAUGES SHOWN ON PLANS ARE BASED ON INSTALLATION OF DECK SUCH THAT IT IS CONTINUOUS OVER A MINIMUM THREE SPAN UNSHORED CONDITION. MANUFACTURER REQUIRED TO VERIFY OTHER SPAN CONDITIONS AND LOADING.
- UNLESS NOTED OTHERWISE, FASTEN STEEL ROOF DECK TO SUPPORTING STRUCTURE ACCORDING TO THE FOLLOWING OPTIONS: (ROOF EXTERIOR ZONES ARE AREAS OF THE ROOF AT "a" DISTANCE FROM THE ROOF PERIMETER AS SPECIFIED IN THE WIND ZONE DIAGRAM LEGEND. ALL OTHER ZONES ARE INTERIOR ZONES.) FASTEN TO SUPPORTS WITH #12 TEK SCREWS IN A 36/7 PATTERN AT EXTERIOR ZONES AND IN A 36/3 PATTERN AT INTERIOR ZONES. FASTEN SIDE LAPS WITH #10 TEK SCREWS AT 6 INCHES ON CENTER AT
  - ROOF PERIMETER, (3) #10 TEK SCREWS PER SPAN AT EXTERIOR ZONES, AND (3) #10 TEK SCREWS PER SPAN AT INTERIOR ZONES. FASTEN TO SUPPORTS WITH PNEUTEK K OR SDK FASTENERS IN A 36/7 PATTERN AT EXTERIOR ZONES AND IN A 36/3 PATTERN AT INTERIOR ZONES. FASTEN SIDE LAPS WITH #10 TEK SCREWS AT 6 INCHES ON
  - CENTER AT ROOF PERIMETER, (3) #10 TEK SCREWS PER SPAN AT EXTERIOR ZONES, AND (3) #10 TEK SCREWS PER SPAN AT INTERIOR ZONES. FASTEN TO SUPPORTS WITH 5/8" PUDDLE WELDS IN A 36/7 PATTERN AT EXTERIOR ZONES AND IN A 36/3
- PATTERN AT INTERIOR ZONES. FASTEN SIDE LAPS WITH #10 TEK SCREWS OR 5/8" PUDDLE WELDS AT 6 INCHES ON CENTER AT ROOF PERIMETER, (3) #10 TEK SCREWS OR (3) 5/8" PUDDLE WELDS PER SPAN AT EXTERIOR ZONES, AND (3) #10 TEK SCREWS OR (3) 5/8" PUDDLE WELDS PER SPAN AT INTERIOR ZONES. PROVIDE WELDING WASHERS WHERE STEEL DECK IS LESS THAN 22 GAGE THICK AND/OR WHERE 5/8" PUDDLE
- WELDS ARE USED. USE WELDING WASHERS WITH A NOMINAL 3/8" DIAMETER HOLE AND A MINIMUM 16 GAGE THICKNESS. SUBMIT SHOP DRAWINGS FOR APPROVAL SHOWING IDENTIFICATION, LAYOUT, CONNECTION DETAILS, AND
- FASTENING FOR STEEL DECK. DO NOT BEGIN FABRICATION UNTIL THE SHOP DRAWINGS ARE COMPLETED AND REVIEWED BY THE STRUCTURAL ENGINEER OF RECORD.

### 8. STRUCTURAL SAWN LUMBER

- PROVIDE STRUCTURAL SAWN LUMBER CONFORMING TO THE FOLLOWING STANDARDS: NATIONAL DESIGN SPECIFICATION (NDS), 2015 EDITION
- UNLESS NOTED OTHERWISE, PROVIDE #2 GRADE SOUTHERN YELLOW PINE OR EQUIVALENT FOR DIMENSIONAL FRAMING LUMBER & VERTICAL STUDS. UNLESS NOTED OTHERWISE, PROVIDE #3 SPF OR #3 HEM-FIR OR EQUIVALENT FOR OTHER MISCELLANEOUS FRAMING/BLOCKING.
- ALL LUMBER IN CONTACT WITH CONCRETE OR STEEL FRAMING SHALL BE PROTECTED OR PRESSURE TREATED IN ALL WOOD FASTENINGS MUST CONFORM TO PART 10 OF THE NATIONAL DESIGN SPECIFICATION. BOLTS AND LAG
- SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.1. ALL BOLTS AND LAG SCREWS SHALL BE INSTALLED WITH STANDARD CUT WASHERS. FASTENINGS NOT INDICATED ON PLANS AND DETAILS MUST BE IN ACCORDANCE WITH TABLE 2304.9.1 OF THE
- INTERNATIONAL BUILDING CODE REFERENCED IN STRUCTURAL GENERAL NOTE SECTION 1. UNLESS NOTED OTHERWISE, TOE NAILING AND END NAILING ARE ACCEPTABLE FOR BEARING TYPE CONNECTIONS. PROVIDE METAL CONNECTORS FOR ALL OTHER CONNECTIONS.
- PROVIDE COMMON WIRE NAILS UNLESS NOTED OTHERWISE ON THE DRAWINGS OR RECOMMENDED OTHERWISE BY THE FRAMING CONNECTOR MANUFACTURER.
- FRAMING ACCESSORIES AND STRUCTURAL FASTENERS SHALL BE MANUFACTURED BY SIMPSON COMPANY (OR APPROVED EQUAL) AND OF THE SIZE AND TYPE SHOWN ON THE DRAWINGS. HANGERS NOT SHOWN SHALL BE SIMPSON LUS OF SIZE RECOMMENDED FOR THAT MEMBER BY THE MANUFACTURER. UNLESS SHOWN OTHERWISE, INSTALL MAXIMUM SIZE AND NUMBER OF FASTENERS SHOWN IN THE LATEST SIMPSON CATALOG FOR THE
- CONNECTOR. PROVIDE FASTENERS AND METAL FRAMING HARDWARE WITH A CORROSION RESISTANT METAL OR WITH A MINIMUM G90 GALVANIZED FINISH. FOR METAL IN CONTACT WITH PRESSURE TREATED LUMBER PROVIDE
- STAINLESS STEEL OR G185 GALVANIZED FINISH. UNLESS NOTED OTHERWISE, PROVIDE BLOCKING OR BRIDGING 8'-0" ON CENTER MAXIMUM AND AT ALL BEARING POINTS FOR ALL JOISTS. UNLESS NOTED OTHERWISE, PROVIDE SOLID HORIZONTAL BLOCKING 6'-0" ON CENTER MAXIMUM FOR ALL LOAD
- BEARING STUD WALLS. PROVIDE REINFORCEMENT (WOOD OR STEEL SIDE PIECES) FOR ANY MEMBER CUT FOR THE INSTALLATION OF PLUMBING OR WIRING SUCH THAT THE MEMBER IS OF EQUAL STRENGTH TO THE MEMBER PRIOR TO CUTTING.
  - UNLESS NOTED OTHERWISE, FASTEN MULTIPLE-PLY BEAMS TOGETHER WITH 16D NAILS AT 12 INCHES ON CENTER (STAGGERED).

PROVIDE JOISTS AND RAFTERS CUT TO HAVE HORIZONTAL CONTACT FOR THE FULL WIDTH OF THE SUPPORTING

#### 9. PLYWOOD PLYWOOD PANELS SHALL CONFORM TO THE REQUIREMENTS OF "U.S. PRODUCT STANDARD PS-1 FOR

- CONSTRUCTION AND INDUSTRIAL PLYWOOD" OR AMERICAN PLYWOOD ASSOCIATION (APA) PRP-108 PERFORMANCE STANDARDS.
- STORE STRUCTURAL SHEATHING IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. UNLESS OTHERWISE NOTED, PANELS SHALL BE APA RATED SHEATHING, EXPOSURE 1, WITH NOMINAL THICKNESS & SPAN RATINGS MEETING THE FOLLOWING MINIMUM REQUIREMENTS:
  - 7/16" = .437" THICKNESS 24/16 SPAN RATING 15/32" = .469" THICKNESS - 32/16 SPAN RATING
  - 19/32" = .594" THICKNESS 40/20 SPAN RATING
  - 23/32" = .719" THICKNESS 48/24 SPAN RATING
- ALL SUB-FLOORING AND WALL SHEATHING SHALL BE INSTALLED WITH THE LONG DIMENSION PERPENDICULAR TO THE SUPPORTS. STAGGER ENDS OF ADJACENT PANELS 4'-0" (PANEL END SHALL COINCIDE WITH CENTER OF
- ALL NAILS SHALL BE RING SHANK NAILS U.N.O. PLYWOOD WALL SHEATHING SHALL BE FULL HEIGHT SHEATHING THAT EXTENDS FROM THE BOTTOM SILL PLATE
- TO THE UPPER MOST MEMBER IN THE TOP PLATE.
- WALL SHEATHING MAY BE INSTALLED HORIZONTALLY OR VERTICALLY AS LONG AS HORIZONTAL EDGES ARE FULLY BLOCKED WITH FRAMING LAID FLAT AGAINST THE BACK SIDE OF SHEATHING FOR ATTACHMENT. A MINIMUM GAP OF 1/8" @ ALL SHEATHING JOINTS MUST BE MAINTAINED.

#### 10. PRE-ENGINEERED METAL BUILDING:

- THE PRE-ENGINEERED METAL BUILDING SHALL CONSIST OF ROOF DECK, RIGID FRAMES, METAL WALL PANELS ON FRAMING, CANOPY FRAMING, GUTTERS AND DOWNSPOUTS, AND FLASHING. THE DESIGN SHALL BE IN ACCORDANCE TO AISC AND AISI SPECIFICATIONS AND MBMA "METAL BUILDING
- SYSTEMS MANUAL" DESIGN PRACTICES. METAL BUILDING SYSTEM TO BE DESIGNED TO SUPPORT THE SELFWEIGHT OF THE METAL BUILDING SYSTEM, 10 PSF SUPERIMPOSED DEAD LOAD, AND INDICATED LIVE LOAD AND WIND LOAD ON THESE PLANS. FOR UPLIFT
- CONSIDERATIONS, THE RESISTING DEAD LOAD SHALL BE THE SELFWEIGHT OF THE METAL BUILDING ONLY. THE MANUFACTURER SHALL BE REGULARLY ENGAGED IN METAL BUILDING DESIGN AND MANUFACTURING. CURRENT MBMA MEMBERS ARE APPROVED. OTHERS SHALL SUBMIT PRODUCT DATA FOR REVIEW. SHOP DRAWINGS AND A LETTER OF CERTIFICATION SHALL BE SUBMITTED FOR REVIEW AND APPROVAL PRIOR TO
- FABRICATION. SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF GEORGIA. SHOP DRAWINGS SHALL INDICATE THE DESIGN LOADS AND JOB NAME AND NUMBER. THEY SHALL INCLUDE DRAWINGS OF THE FRAMING MEMBERS WITH THE CONNECTIONS, THE ANCHOR BOLT PLAN AND REACTIONS. STANDARD CUT SHEETS OF THE ABOVE ARE NOT ACCEPTABLE. STANDARD CUT SHEETS MAY BE SUBMITTED FOR SECONDARY FRAMING CONNECTION DETAILS, FLASHING AND SHEETING DETAILS, ETC.

#### **DESIGN LOADS**

- LIVE LOADS: ROOF 20 FLOOR 50
- DEAD LOADS: ROOF 15 FLOOR 50
- WIND DESIGN DATA BASIC WIND SPEED (3 SECOND GUST):
  - RISK CATEGORY WIND EXPOSURE INTERNAL PRESSURE COEFF INTERNAL PRESSURE COEFF
- COMPONENTS AND CLADDING PRESSURES: SEISMIC DESIGN DATA:
  - SEISMIC IMPORTANCE FACTOR: I = 1.0RISK CATEGORY CATEGORY II MAPPED SPECTRAL RESPONSE ACCELERATIONS: S<sub>S</sub>= 0.199  $S_1 = 0.093$
  - SITE CLASS: SITE CLASS D SPECTRAL RESPONSE COEFFICIENTS:  $S_{DS} = 0.212$  $S_{D1} = 0.149$
  - SEISMIC DESIGN CATEGORY: CATEGORY C BASIC SEISMIC FORCE RESISTING SYSTEM: INTERMEDIATE REINFORCED MASONRY SHEAR
  - (RESPONSE MODIFICATION FACTOR) (R=3.5)SEISMIC RESPONSE COEFFICIENT:  $C_S = 0.061$
  - DESIGN BASE SHEAR: V= 22 KIPS ANALYSIS PROCEDURE: **EQUIVALENT LATERAL FORCE PROCEDURE**

115 MPH

CATEGORY II

EXPOSURE C

±0.00 OPEN BUILDING

SEE TABLE THIS SHEET

±0.18 ENCLOSED (PROTECTED OPENINGS)

#### **INDEX OF DRAWINGS** ISSUED DATE REV **REV DATE** NUMBER SHEET NAME STRUCTURAL GENERAL NOTES METAL BUILDING FOUNDATION PLAN 06/18/19 SLAB FOUNDATION PLAN 02/21/20 FRAMING PLAN 05 FEB 2020 15 MAR 2019 **BUILDING SECTIONS** BUILDING SECTIONS 03/12/20 S3.03 BUILDING SECTIONS 03/12/20 ELEVATIONS 15 MAR 2019 STRUCTURAL DETAILS 18 JUN 2019

REINF	ORCE	MENT	LAP S	PLICE	:S (CL	ASS A	, INCI	HES)	
REINFORCEMENT	CONCR	ETE				f'c =	= 3000 I	PSI @ 2	8 DAY
REINFORCING			1	REINFO	RCEMEI	NT SIZE			
LOCATION	#3 GR40	#4	#5	#6	#7	#8	#9	#10	#11
ТОР	13	25	31	37	54	62	70	79	87
OTHER	12	19	24	29	42	48	54	61	67

REINF	ORCE	<b>4ENT</b>	LAP S	PLICE	S (CL	ASS B	, INC	HES)	
REINFORCEMENT	CONCR	ETE				f'c =	3000 F	PSI @ 28	B DAYS
REINFORCING LOCATION	REINFORCEMENT SIZE								
	#3 GR40	#4	#5	#6	#7	#8	#9	#10	#11
ТОР	17	33	41	49	71	81	91	103	114
OTHER	16	25	32	38	55	63	71	80	88

DEVELOPME	NT LE	NGTH	OF S	TAND	ARD H	OOKS	(INC	HES)
REINFORCEMENT CONCRETE					f'c =	3000 P	SI @ 28	DAYS
REINFORCING LOCATION			I	REINFO	RCEMEN	NT SIZE		
	#3 GR40	#4	#5	#6	#7	#8	#9	#10
ТОР	8	10	12	15	17	19	22	25

# REINFORCEMENT LAP SPLICES (8" CONCRETE

MASONRY UN	111)					
	f'm :	= 1,500	PSI , f	'y = 60	KSI, &	Ø=0.8
REINFORCING			l	REINFO	RCEME	NT SIZI
LOCATION	#3	#4	#5	#6	#7	#8
CENTER OF CELL	19	25	31	57	79	113
K = 1 1/2"	24	42	65	131	178	248

ROOF				RO	OOF OVERHA	EXTERIOR WALLS		
EFFECTIVE WIND AREA	ZONE 1	ZONE 2	ZONE 3	ZONE 1 OVHG	ZONE 2 OVHG	ZONE 3 OVHG	ZONE 4	ZONE 5 (NOTE 2)
10 SF	+16.0 PSF	+23.8 PSF	+23.8 PSF	+16.0 PSF	+16.0 PSF	+16.0 PSF	+21.8 PSF	+21.8 PSF
	-23.8 PSF	-39.9 PSF	-39.9 PSF	-37.9 PSF	-37.9 PSF	-60.1 PSF	-23.6 PSF	-29.0 PSF
25 SF	+16.0 PSF	+22.4 PSF	+22.4 PSF	+16.0 PSF	+16.0 PSF	+16.0 PSF	+20.5 PSF	+20.5 PSF
25 51	-23.0 PSF	-34.3 PSF	-34.3 PSF	-37.1 PSF	-37.1 PSF	-44.0 PSF	-22.3 PSF	-26.5 PSF
50 SF	+16.0 PSF	+21.3 PSF	+21.3 PSF	+16.0 PSF	+16.0 PSF	+16.0 PSF	+19.5 PSF	+19.5 PSF
30 31	-22.4 PSF	-30.1 PSF	-30.1 PSF	-36.5 PSF	-36.5 PSF	-31.9 PSF	-21.3 PSF	-24.6 PSF
100 SF	+16.0 PSF	+20.2 PSF	+20.2 PSF	+16.0 PSF	+16.0 PSF	+16.0 PSF	+18.6 PSF	+18.6 PSF
100 31	-21.8 PSF	-25.8 PSF	-25.8 PSF	-35.9 PSF	-35.9 PSF	-19.8 PSF	-20.4 PSF	-22.6 PSF
500 SF	+16.0 PSF	+17.7 PSF	+17.7 PSF	+16.0 PSF	+16.0 PSF	+16.0 PSF	+16.3 PSF	+16.3 PSF
300 SF	-21.8 PSF	-25.8 PSF	-25.8 PSF	-25.8 PSF	-25.8 PSF	-19.8 PSF	-18.1 PSF	-18.1 PSF

### **WIND PRESSURE SCHEDULE NOTES:**

- (+) AND (-) SIGNES INDICATE PRESSURES ACTING TOWARD AND AWAY FROM THE BUILDING SURFACE.
- PRESSURES APPLY a = 3'-0" FROM PROMINENT BUILDING CORNER IN EACH DIRECTION. ALL WIND PRESSURE VALUES IN POUNDS PER SQ. FT(PSF) & ARE CONSIDERED ULTIMATE VALUES. MULTIPLY
- VALUES BY 0.6 FOR NOMINAL PRESSURES.



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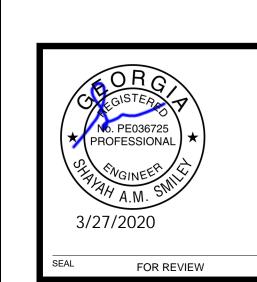
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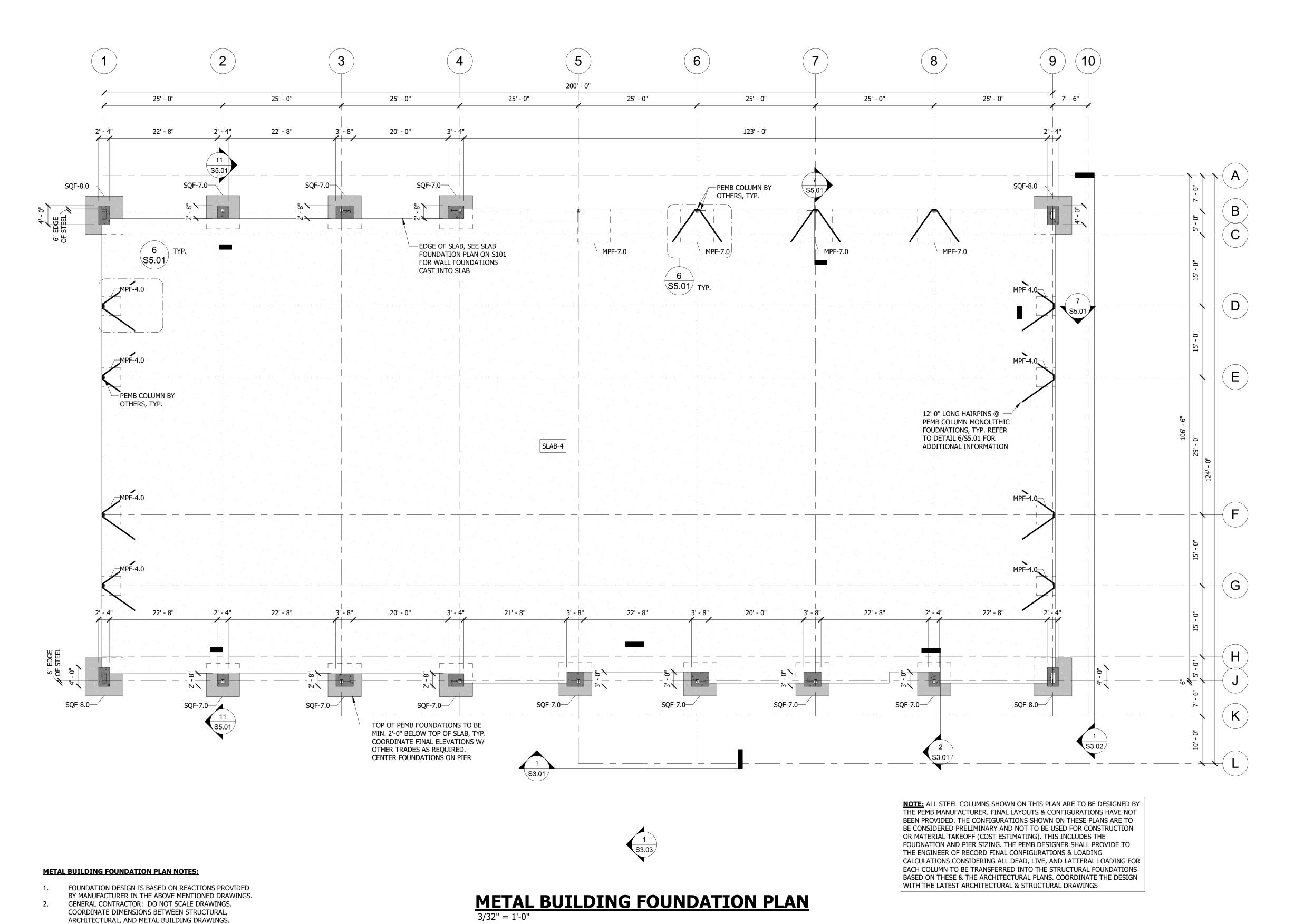
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CHURCH



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**STRUCTURAL GENERAL NOTES** 



NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY

VERIFY ELEVATIONS OF ALL STRUCTURAL FOUNDATIONS, AND SLABS INDICATED ON THESE PLANS W/ DRAWINGS FROM OTHER DISCIPLINES BEFORE COMMENCING

"XXF-X.X" INDICATES CONCRETE FOUNDATION. SEE SCHEDULE THIS SHEET FOR SIZE, TYPE, & REINFORCING

STRUCTURAL CONCRETE PIERS WHERE INDICATED BELOW PEMB FRAMES. SEE CROSS-SECTION DETAIL THIS SHEET FOR PIER REINFORCING. SEE DETAILS ON S501 FOR PIER TO FOUNDATION CONNECTION AND ANCHOR BOLT

ATTACHMENT & EMBEDMENT LENGTHS AT METAL BUILDING

DISCREPANCIES BEFORE PROCEEDING WITH WORK.

CONSTRUCTION.

REQUIREMENTS.

LOCATIONS.



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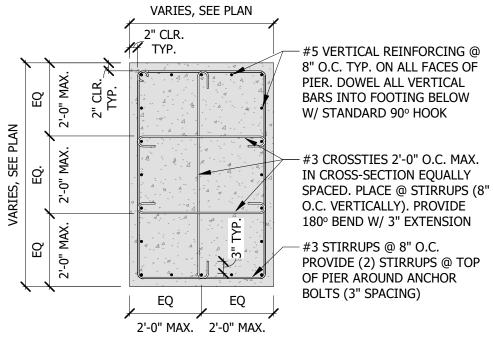
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GA LICENSED CONSULTING ENGINEERS

	<b>DRAWING LEGEND</b>
MARK	DESCRIPTION
	6" METAL STUD WALL. SEE METAL STUD WALL SCHEDULE
	FLOOR/ROOF JOIST. SEE PLAN FOR TYPE & SIZE
	STRUCTURAL BEAM/HEADERS
	SPAN DIRECTION OF FLOOR/ROOF SHEATHING.
	STEP IN WALL OR FOUNDATION HEIGHT
•	1/2" Ø THREADED ROD
C.J.	CONTROL JOINT
7771	STEP IN SLAB ELEVATION
	CONCRETE PIER

HEADERS SHOWN ON PLANS ARE BELOW FLOOR OR ROOF FRAMING FOR THAT LEVEL. POSTS SHOULD BE INSTALLED BELOW ALL MULTI-PLY BEAMS OR GIRDERS DIRECTLY BELOW THE POINT OF BEARING (POST WIDTH TO MATCH BEAM/GIRDER WIDTH). POSTS SHALL PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WHERE POSSIBLE.



# **1 CONCRETE PIER SECTION** 1/2" = 1'-0"

	<b>FOUNDATION SCHEDULE</b>
MARK	DESCRIPTION
IMF-2.0	2' - 0" WIDE x 1' - 0" DEEP CONTINUOUS INTERIOR MONOLITHIC WALL FOUNDATION. PROVIDE (3) #5 BARS BOTTOM CONTINUOUS & #3 TRANSVERSE BARS @ 24" O.C.
MPF-4.0	4'-0" WIDE SQUARE x 1 ' -6" DEEP MONOLITHIC PAD FOUNDATION @ PEMB COLUMNS W/ (6) #5 BARS EACH WAY BOTTOM
MPF-7.0	7' - 0" WIDE SQUARE x 1' - 6" DEEP MONOLITHIC PAD FOUNDATION @ PEMB COLUMNS W/ (8) #5 BARS EACH WAY BOTTOM
SLAB-4	4" THICK CONCRETE SLAB ON GRADE W/ 6x6 W1.4x1.4 W.W.F OVER 6 MIL. VAPOR BARRIER ON CLEAN COMPACTED FILL
SQF-4.0	4' - 0" SQUARE x 1' - 0" DEEP CONCRETE PAD FOUNDATION W/ (5) #5 BOTTOM BARS EACH WAY
SQF-7.0	7' - 0" SQUARE x 1' - 0" DEEP CONCRETE PAD FOUNDATION W/ (8) #5 BOTTOM BARS EACH WAY
SQF-8.0	8' - 0" SQUARE x 1' - 0" DEEP CONCRETE PAD FOUNDATION W/ (10) #5 BOTTOM BARS EACH WAY
TDF-2.0	2' - 0" WIDE x 1' - 6" DEEP TURN DOWN SLAB FOUNDATION. PROVIDE (3) #5 CONTINUOUS BOTTOM BARS & #3 TRANSVERSE BARS @ 24" O.C.



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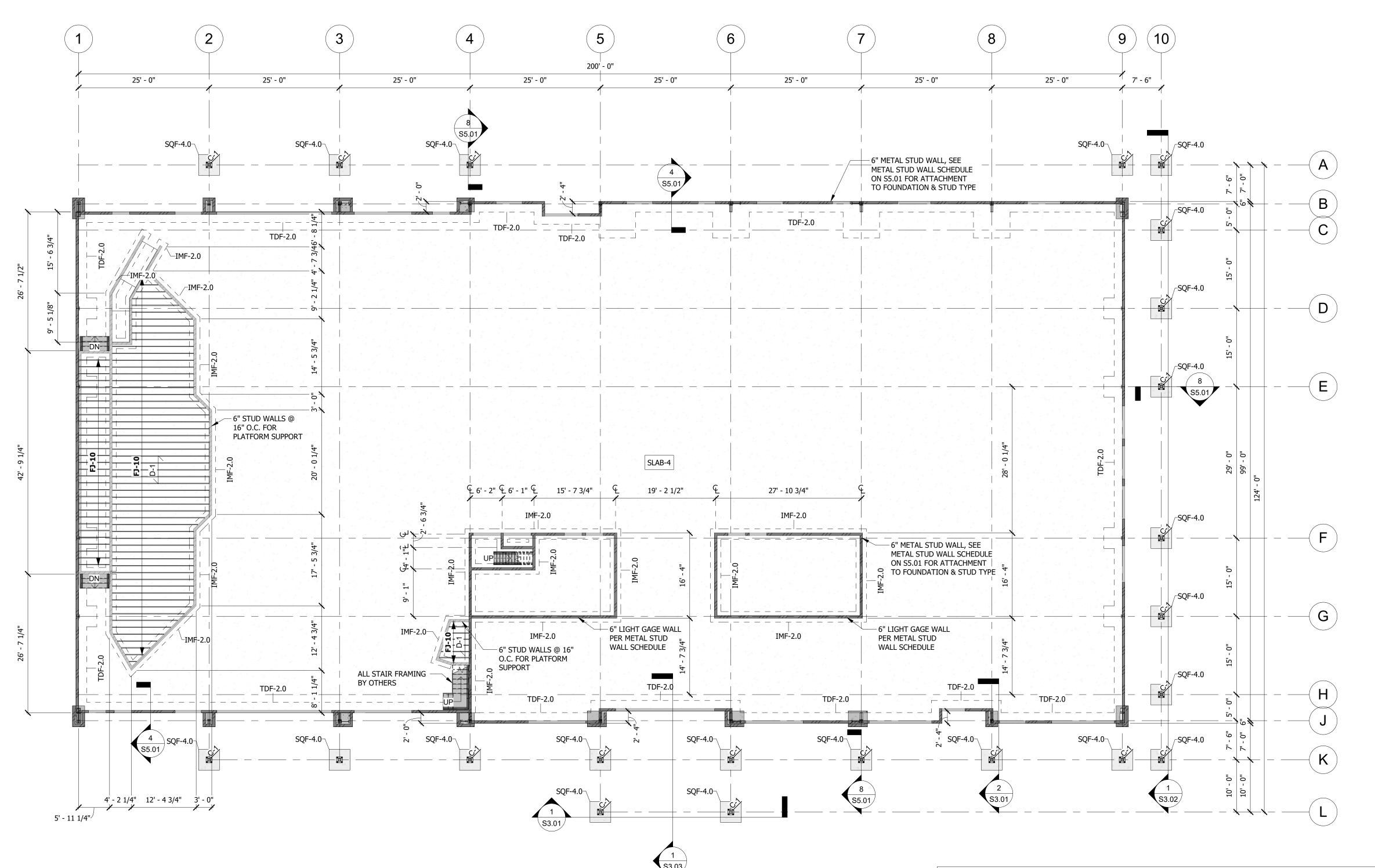
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DISTRIBUTION

METAL BUILDING **FOUNDATION** PLAN

DATE

2020.03.23



#### **FOUNDATION PLAN NOTES:** SEE ARCHITECTURAL DRAWINGS FOR SLOPES, DROPS, AND

- DRAIN LOCATIONS IN SLABS. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS OF ALL OPENINGS. VERIFY/COORDINATE SILL HEIGHTS AND DETAILS OF WALL OPENINGS WITH ARCHITECTURAL DRAWINGS.
- GENERAL CONTRACTOR: DO NOT SCALE DRAWINGS. COORDINATE DIMENSIONS BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- VERIFY ELEVATIONS OF ALL STRUCTURAL FOUNDATIONS AND FLOORS INDICATED ON THESE PLANS W/ ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. "XXF-X.X" INDICATES CONCRETE FOUNDATION. SEE SCHEDULE THIS SHEET FOR SIZE, TYPE, & REINFORCING
- REQUIREMENTS. INDICATES STRUCTURAL COLUMN. SEE COLUMN SCHEDULE THIS SHEET FOR COLUMN TYPE &
- DESIGN. INDICATES 23/32" THICK OSB/PLYWOOD
- FLOOR SHEATHING.
  - a. EDGE NAILING: 6" O.C. MAX.
  - b. FIELD NAILING: 12" O.C. MAX.

# **FOUNDATION PLAN**

METAL STUD WALL SCHEDULE DESCRIPTION METAL STUD TYPE & **BOTTOM TRACK &** TOP TRACK & ATTACHMENT ATTACHMENT SPACING 600T250-54 (50) KSI DEFLECTION 600T125-43 (33) KSI W/ (1) #12 TRACK W/ (1) #12 TEK SCREW @ 6" WIDE METAL STUD WALL | 600S200-43 (33) KSI | TEK SCREW @ EACH FLANGE & (2) EACH FLANGE. ATTACH TRACK TO HILTI X-U PAF @ 16" O.C., 1-1/4" @ 16" O.C. (ALL HEIGHTS) STEEL W/ (2) HILTI X-U PAF @ 12" EMBEDMENT INTO FOUNDATION

PROVIDE LATERAL BRACING TO WALL STUDS @ 8' - 0" O.C.

SEE METAL STUD FRAMING DETAILS ON SHEET S5.01 FOR ADDITIONAL INFORMATION NOT DESCRIBED HERE.



INC.

	DRAWING LEGEND
MARK	DESCRIPTION
	6" METAL STUD WALL. SEE METAL STUD WALL SCHEDULE
	FLOOR/ROOF JOIST. SEE PLAN FOR TYPE & SIZE
	STRUCTURAL BEAM/HEADERS
	SPAN DIRECTION OF FLOOR/ROOF SHEATHING.
V	STEP IN WALL OR FOUNDATION HEIGHT
•	1/2" Ø THREADED ROD
C.J.	CONTROL JOINT
7771	STEP IN SLAB ELEVATION
	CONCRETE PIER

### HEADERS SHOWN ON PLANS ARE BELOW FLOOR OR ROOF

FRAMING FOR THAT LEVEL. POSTS SHOULD BE INSTALLED BELOW ALL MULTI-PLY BEAMS OR GIRDERS DIRECTLY BELOW THE POINT OF BEARING (POST WIDTH TO MATCH BEAM/GIRDER WIDTH). POSTS SHALL PROVIDE CONTINUOUS LOAD PATH TO FOUNDATION WHERE POSSIBLE.

<b>RAFTER/JOIST LEGEND</b>				
MARK	DESCRIPTION	BEARING CLIP		
FJ-10	800S162-54 (50KSI) FLOOR JOISTS @ 16" O.C.	LS687		

	<b>FOUNDATION SCHEDULE</b>
MARK	DESCRIPTION
IMF-2.0	2' - 0" WIDE x 1' - 0" DEEP CONTINUOUS INTERIOR MONOLITHIC WALL FOUNDATION. PROVIDE (3) #5 BARS BOTTOM CONTINUOUS & #3 TRANSVERSE BARS @ 24" O.C.
MPF-4.0	4'-0" WIDE SQUARE x 1 ' -6" DEEP MONOLITHIC PAD FOUNDATION @ PEMB COLUMNS W/ (6) #5 BARS EACH WAY BOTTOM
MPF-7.0	7' - 0" WIDE SQUARE x 1' - 6" DEEP MONOLITHIC PAD FOUNDATION @ PEMB COLUMNS W/ (8) #5 BARS EACH WAY BOTTOM
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SQF-4.0	4' - 0" SQUARE x 1' - 0" DEEP CONCRETE PAD FOUNDATION W/ (5) #5 BOTTOM BARS EACH WAY
SQF-7.0	7' - 0" SQUARE x 1' - 0" DEEP CONCRETE PAD FOUNDATION W/ (8) #5 BOTTOM BARS EACH WAY
SQF-8.0	8' - 0" SQUARE x 1' - 0" DEEP CONCRETE PAD FOUNDATION W/ (10) #5 BOTTOM BARS EACH WAY
TDF-2.0	2' - 0" WIDE x 1' - 6" DEEP TURN DOWN SLAB FOUNDATION. PROVIDE (3) #5 CONTINUOUS BOTTOM BARS & #3 TRANSVERSE BARS @ 24" O.C.

	STEEL CO	LUMN SCHEDULE
MARK	DESCRIPTION	BASE PLATE
C-1	HSS6x6x1/2	14"x14"x3/4" THICK STEEL BASE PLATE W/ (4) 3/4" x 1'-0" LONG ANCHOR BOLT

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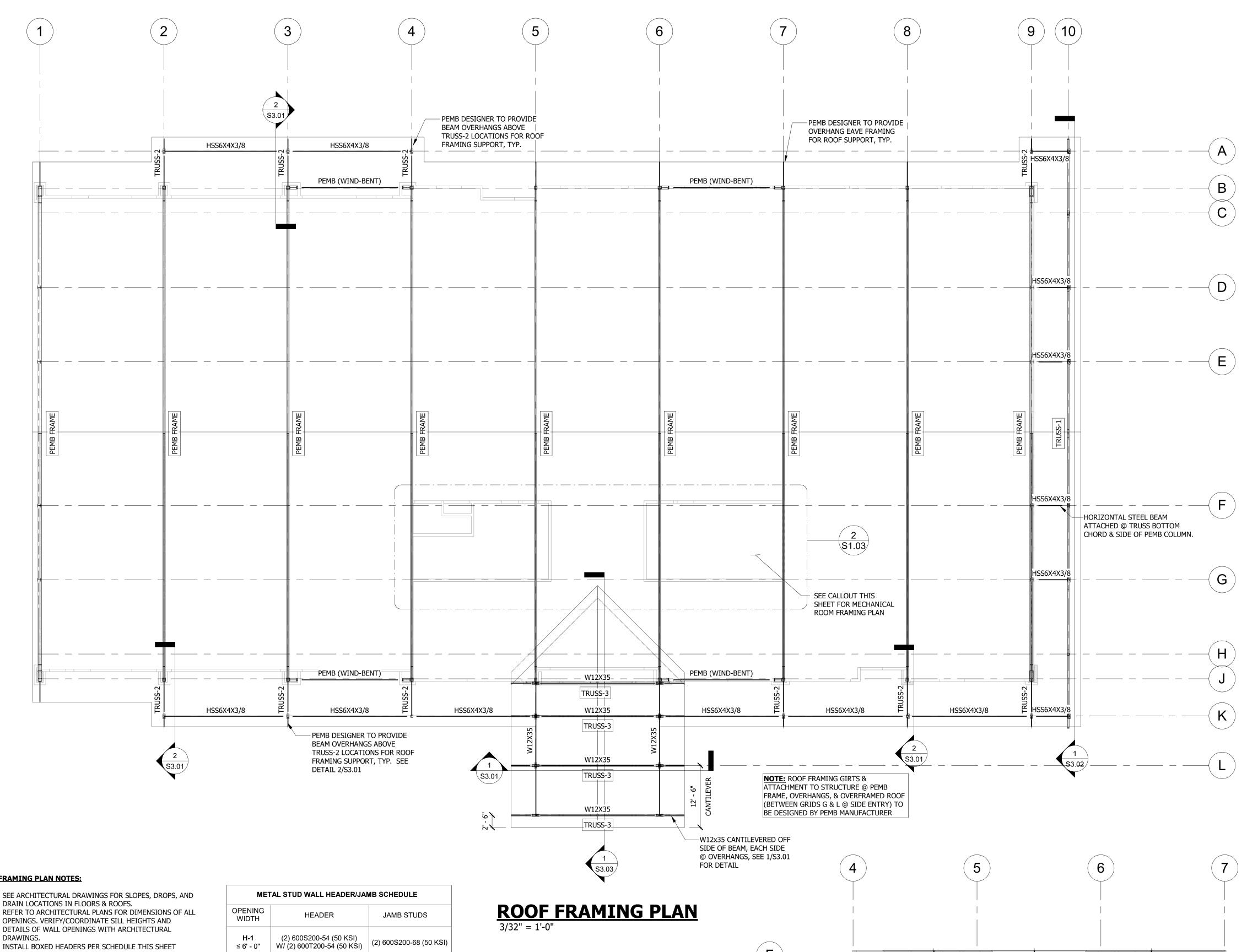
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SLAB **FOUNDATION** PLAN



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**ARCHITECTS** 

ARCHITECT OF RECORD

DAVID R GOODSPEED, AIA

	<b>DRAWING LEGEND</b>
MARK	DESCRIPTION
	6" METAL STUD WALL. SEE METAL STUD WALL SCHEDULE
	FLOOR/ROOF JOIST. SEE PLAN FOR TYPE & SIZE
	STRUCTURAL BEAM/HEADERS
	SPAN DIRECTION OF FLOOR/ROOF SHEATHING.
	STEP IN WALL OR FOUNDATION HEIGHT
•	1/2" Ø THREADED ROD
C.J.	CONTROL JOINT
7771	STEP IN SLAB ELEVATION
	CONCRETE PIER

### HEADERS SHOWN ON PLANS ARE BELOW FLOOR OR ROOF

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RAFTER/JOIST LEGEND				
MARK	DESCRIPTION	BEARING CLIP		
FJ-10	800S162-54 (50KSI) FLOOR JOISTS @ 16" O.C.	LS687		

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FRAMING PLAN

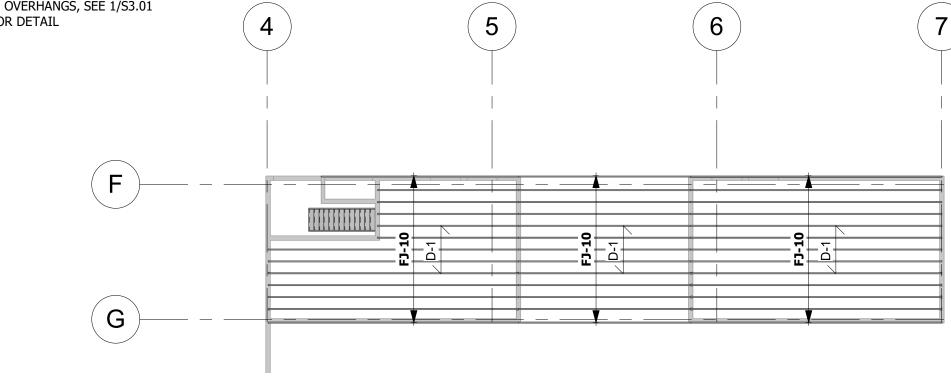
### **ROOF FRAMING PLAN NOTES:**

- SEE ARCHITECTURAL DRAWINGS FOR SLOPES, DROPS, AND
- DRAIN LOCATIONS IN FLOORS & ROOFS. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS OF ALL OPENINGS. VERIFY/COORDINATE SILL HEIGHTS AND DETAILS OF WALL OPENINGS WITH ARCHITECTURAL
- INSTALL BOXED HEADERS PER SCHEDULE THIS SHEET ABOVE ALL OPENINGS IN METAL STUD WALL. VERIFY
- LOCATIONS OF OPENINGS W/ ARCHITECTURAL DRAWINGS. SEE WALL STUD SCHEDULE THIS SHEET FOR EXTERIOR WALL STUD SIZES & ATTACHMENTS.
- GENERAL CONTRACTOR: DO NOT SCALE DRAWINGS. COORDINATE DIMENSIONS BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS. NOTIFY STRUCTURAL ENGINEER AND ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH WORK.
- INDICATES 23/32" THICK OSB/PLYWOOD FLOOR SHEATHING.
  - EDGE NAILING: 6" O.C. MAX. FIELD NAILING: 12" O.C. MAX.
- INDICATES METAL ROOF DECK (1.5B20). FASTENER PATTERN SHALL BE 36/3/3 AS FOLLOWS: a. END LAPS: (3) #12-24 x 1 1/4" LONG FASTENERS AT EACH 36" WIDE SHEET
  - SIDE LAPS: (3) #12 X 3/4" LONG FASTENERS PER SPAN

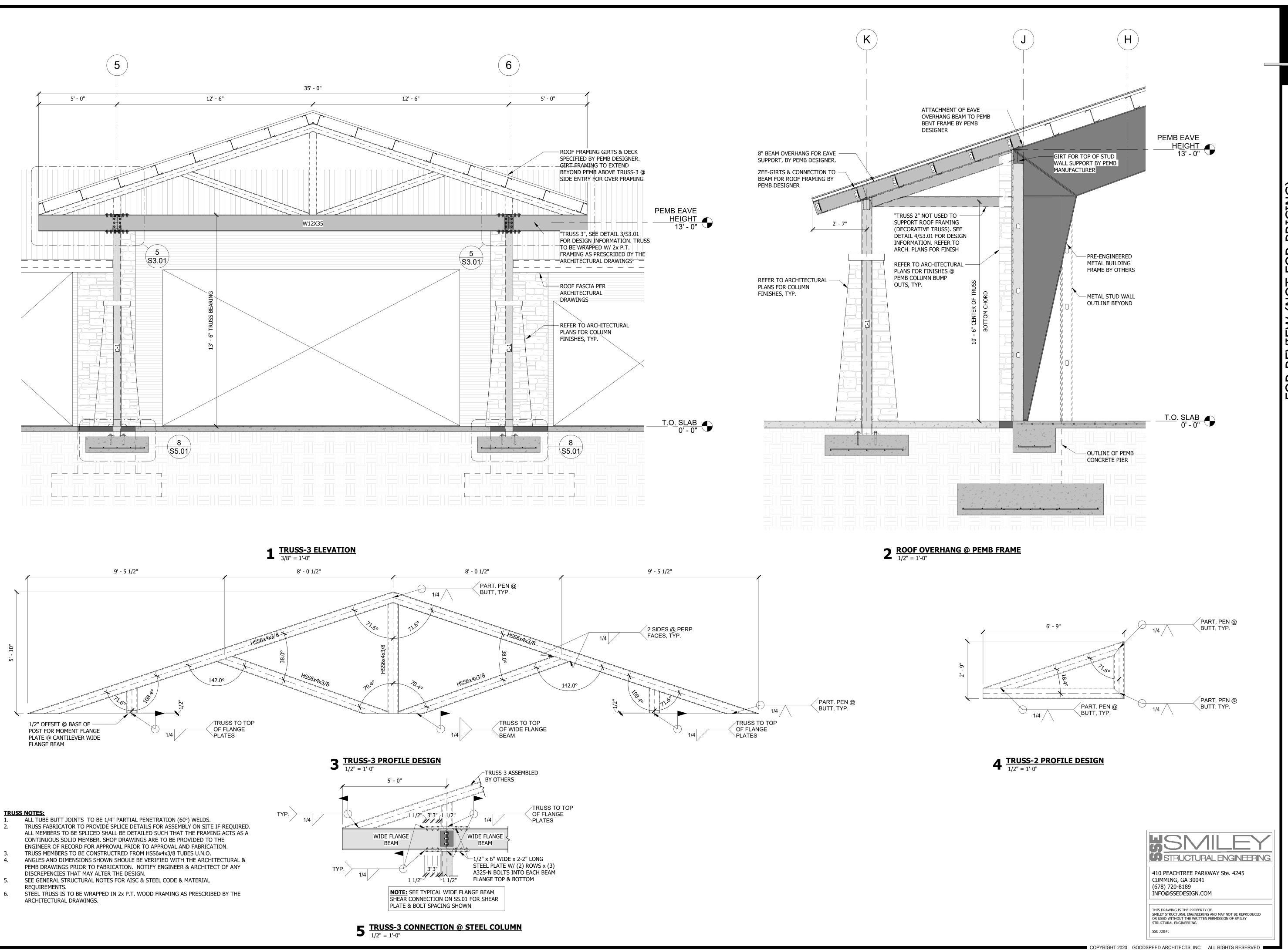
# (2) 800S200-54 (50 KSI) (2) 600S162-68 (50 KSI) W/ (2) 600T200-54 (50 KSI) (2) 1200S200-97 (50 KSI) (3) 600S200-97 (50 KSI) W/(2) 600T200-68 (50 KSI)

### PROVIDE 600T200-54 (50 KSI) SILL BELOW ALL WINDOW

- OPENINGS. ATTACH JAMB BOTTOMS PER DETAIL 5/S5.01 ATTACH JAMB TOPS W/ (2) CLARKDIETRICH FASTTOP CLIPS FTC5 WITH (3) PROPRIETARY SCREWS TO STUD
- AND (2) HILTI X-U PAF TO THE STEEL. ATTACH HEADER TO JAMB PER DETAIL 9/S5.01. SEE METAL STUD FRAMING DETAILS ON SHEET S/5.01 FOR ADDITIONAL INFORMATION NOT DESCRIBED



MECHANICAL ROOM FLOOR FRAMING PLAN





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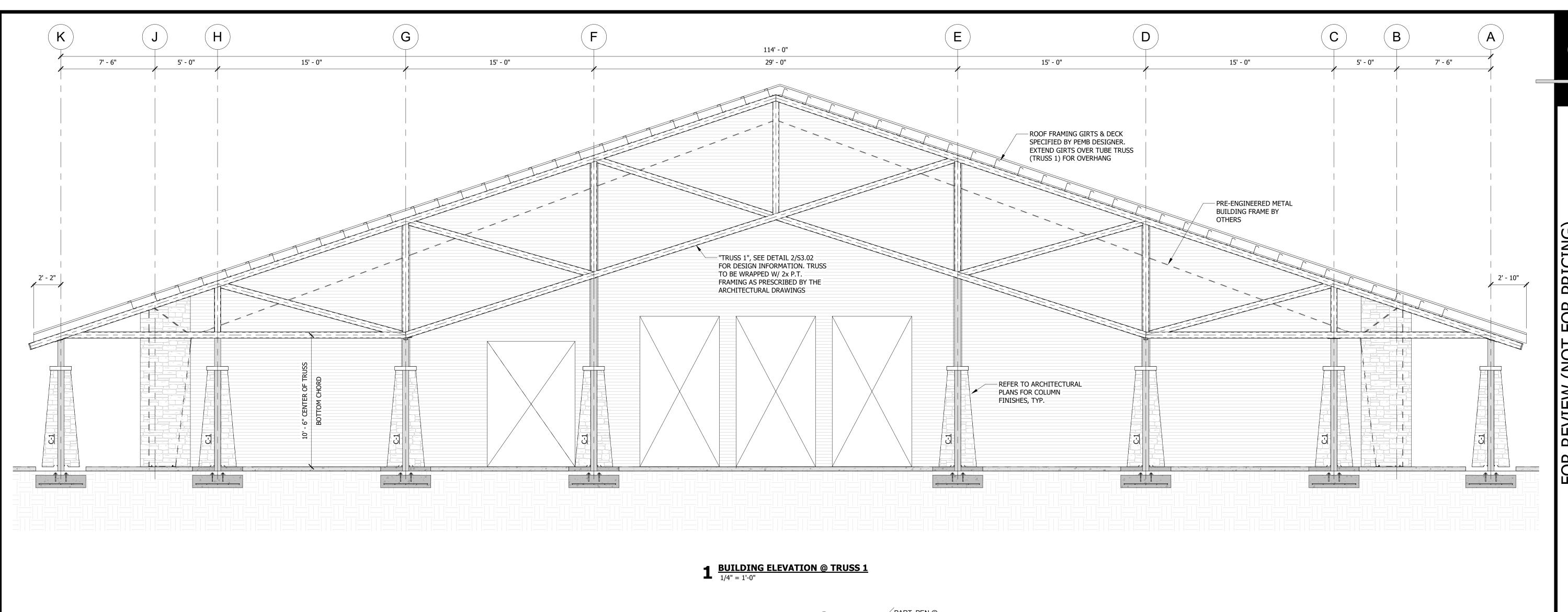
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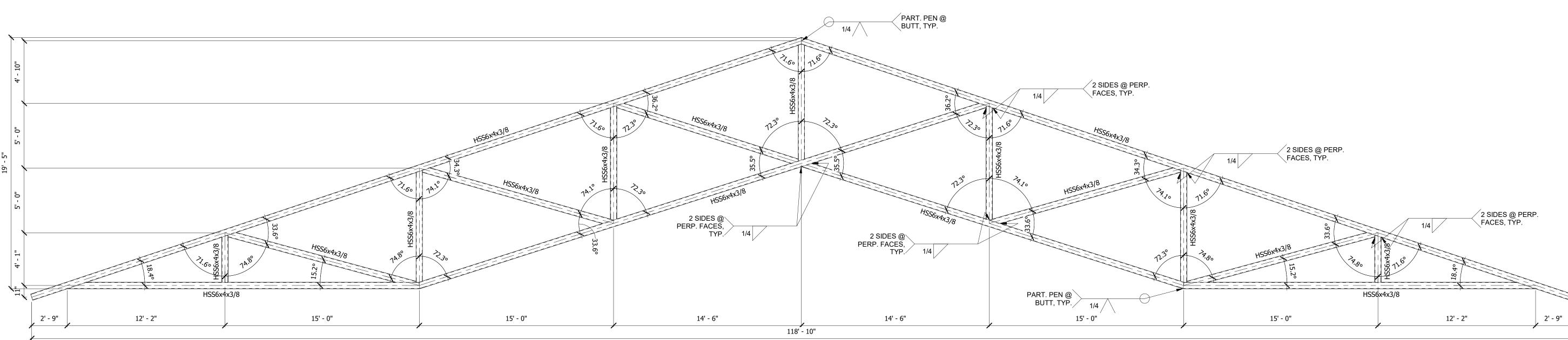
HIGHWAY 76 BLAIRSVILLE, GEORGIA





BUILDING **SECTIONS** 





# 2 $\frac{\text{TRUSS 1 PROFILE DESIGN}}{1/4" = 1'-0"}$

#### ALL TUBE BUTT JOINTS TO BE 1/4" PARTIAL PENETRATION (60°) WELDS. TRUSS FABRICATOR TO PROVIDE SPLICE DETAILS FOR ASSEMBLY ON SITE IF REQUIRED.

- ALL MEMBERS TO BE SPLICED SHALL BE DETAILED SUCH THAT THE FRAMING ACTS AS A CONTINUOUS SOLID MEMBER. SHOP DRAWINGS ARE TO BE PROVIDED TO THE ENGINEER OF RECORD FOR APPROVAL PRIOR TO APPROVAL AND FABRICATION.
- TRUSS MEMBERS TO BE CONSTRUCTRED FROM HSS6x4x3/8 TUBES U.N.O. ANGLES AND DIMENSIONS SHOWN SHOULE BE VERIFIED WITH THE ARCHITECTURAL & PEMB DRAWINGS PRIOR TO FABRICATION. NOTIFY ENGINEER & ARCHITECT OF ANY DISCREPENCIES THAT MAY ALTER THE DESIGN.
- SEE GENERAL STRUCTURAL NOTES FOR AISC & STEEL CODE & MATERIAL
- STEEL TRUSS IS TO BE WRAPPED IN 2x P.T. WOOD FRAMING AS PRESCRIBED BY THE ARCHITECTURAL DRAWINGS.

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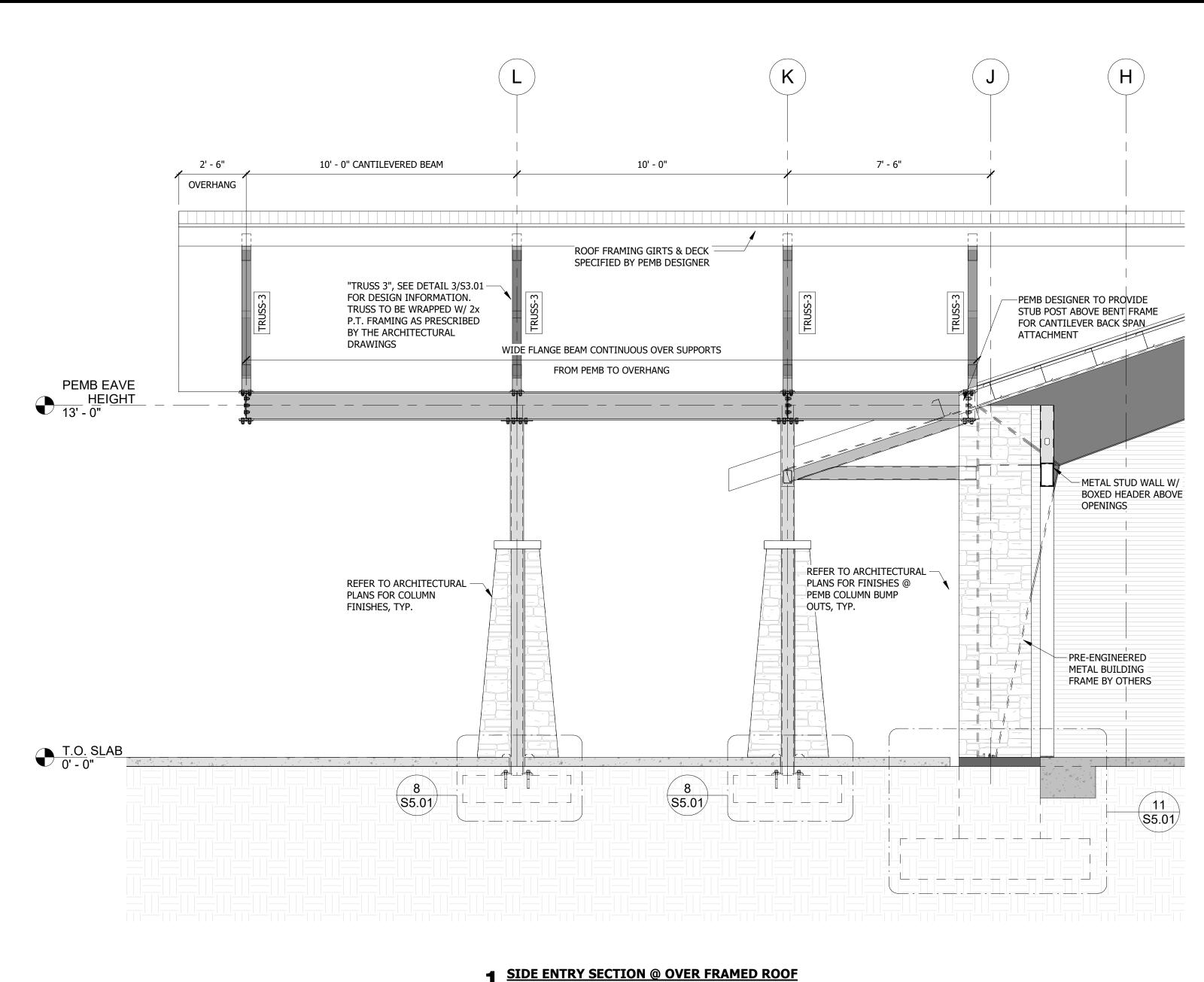
## GRACE PCA PRESBYTERIAN CHURCH

HIGHWAY 76 BLAIRSVILLE, GEORGIA



GAI PROJECT NO.	2019.05
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BUILDING SECTIONS



# 1 SIDE ENTRY SECTION @ OVER FRAMED ROOF 3/8" = 1'-0"



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## GRACE PCA PRESBYTERIAN CHURCH

HIGHWAY 76 BLAIRSVILLE, GEORGIA



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BUILDING SECTIONS

STRUCTURAL ENGINEERING

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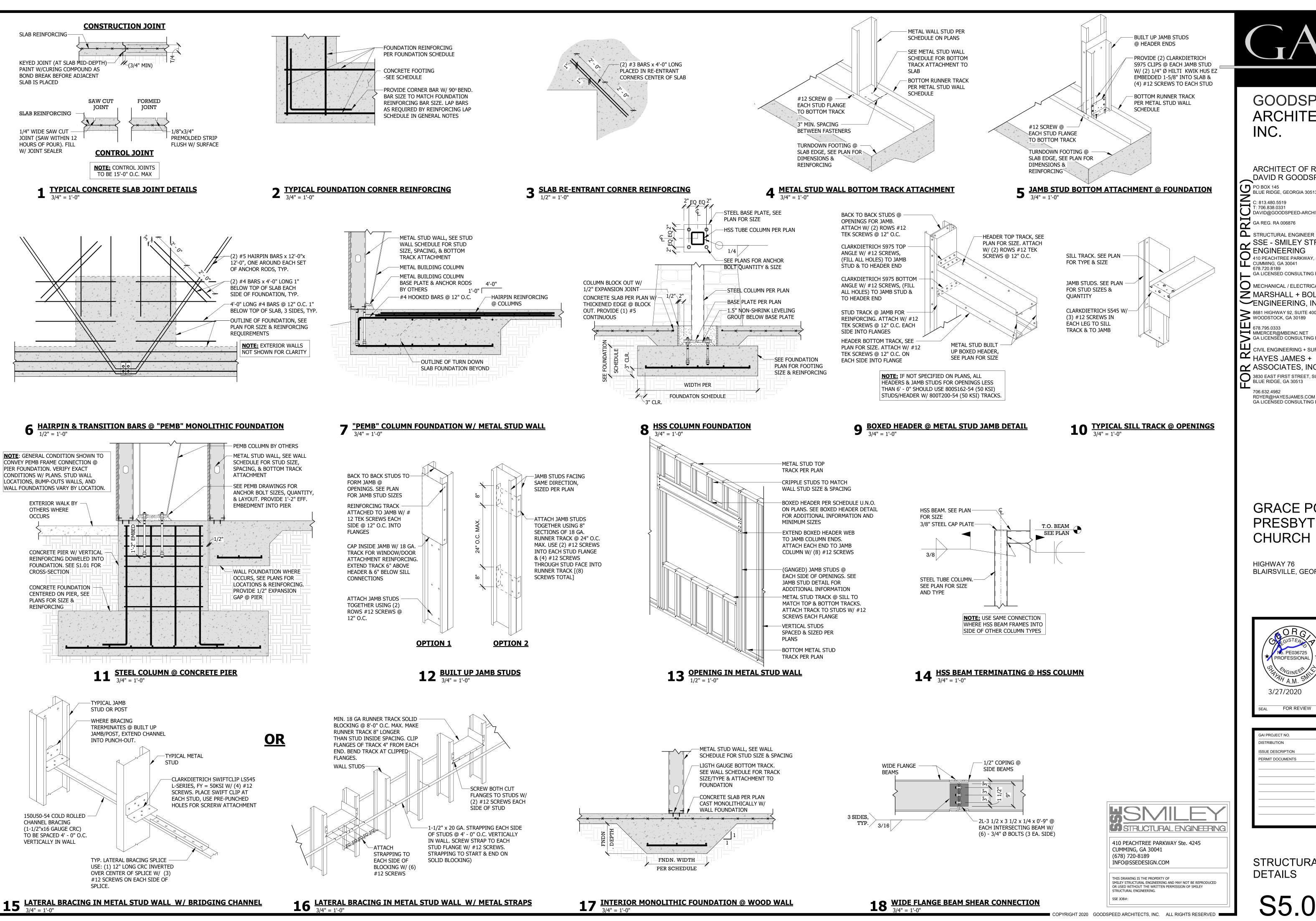
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STRUCTURAL **DETAILS**